

Appendix A.56:

200 Cashmere Rd – VsVp 38171

Table 1: Site Description for 200 Cashmere Road (VsVp 38171).

Attribute	Yes/No			Description/Date	Symbol in Figure 1
	10-m Buffer	20-m Buffer	50-m Buffer		
Near a body of surface water or other free face features?	No	No	No	The center of the site is ~460 m to the NE from the Heathcote River (the free-face height is ~1 m). The center of the site is ~480 m to the NW from the unnamed stream (the free-face height is ~0.5 m).	NA
Lateral spreading observed during the CES?	No	No	No	No lateral spreading was observed by the mapping team. ¹	NA
Nearby buildings or structures?	No	No	Yes	Building coverage of the 50-m buffer is 1%.	NA
Sloping land?	No	No	No	Flat land, open field	NA
Step changes in the ground surface?	No	No	No	NA	NA
Retaining walls?	No	No	No	NA	NA
Vegetation?	No	No	Yes	Trees and bushes cover 1% of the 50-m buffer. They are in NE and SE quadrants of the 50-m buffer.	NA
Anthropogenic changes to the site between the LiDAR surveys?	ND	ND	ND	Not evaluated because LiDAR surveys were not used to estimate the ejecta-induced settlement at the site. Ejecta were evidently absent from the site.	NA
Other important factors?	Yes	Yes	Yes	Paddock is subject to ploughing. Uneven surface and tall grass.	NA

Note: Buffer is the area within a circle of a specified radius with VsVp investigations done at its center (172.608100°, -43.572615°).

¹ Canterbury Geotechnical Database. (2012). "Observed Ground Crack Locations", Map Layer CGD0400 - 23 July 2012, retrieved July 09, 2018 from <https://canterburygeotechnicaldatabase.projectorbit.com/>



Figure 1: Site plan.

Note 1: The LiDAR surveys were not considered in the ejecta-induced settlement assessment because the site had no ejecta.

Table 2: LiDAR flight error adjustments, global adjustments for the difference between average LiDAR point elevations and benchmark survey elevations, and vertical tectonic movement adjustments.

Earthquake Event(s)	LiDAR Flight Error	Adjustments (mm)	
		Global Offset ²	Tectonic Vertical Movement
Sep-10	NA	-3	0
Feb-11	NA	16	-25
Jun-11	0	38	-10
Dec-11	NA	-65	0
CES	NA	-14	-35

Any LiDAR survey affected by ejecta?

No

Note: The negative sign indicates the subtraction from the ground surface subsidence, while the positive sign indicates the addition to the ground surface subsidence.

² Russell, J., & van Ballegooy, S. (2015). *Canterbury Earthquake Sequence: Increased liquefaction vulnerability assessment methodology*. New Zealand: Tonkin & Taylor Ltd.

Table 3: Ejecta-Induced settlement for the top 20 m of the soil profile for the 50th %ile PGA, $P_L=50\%$, and $C_{FC}=0.13$ using BI-2014, ZRB-2002, and I_c cutoff of 2.6.

Earthquake Event(s)	M_w	PGA (g)	Depth to Groundwater (m)	* S_{V1D} (mm)		
				10-m buffer	20-m buffer	50-m buffer
Sep-10	7.1	0.25	0.7	72±20	72±20	72±20
Feb-11	6.2	0.46	1.0	80±50	80±50	80±50
Jun-11	6.2	0.19	0.7	50±25	50±25	50±25
Dec-11	6.1	0.13	0.7	10±50	10±50	10±50

Notes: S_{V1D} = Average vertical settlement due to volumetric compression using Boulanger and Idriss (2014) (BI-2014), Zhang et al. (2002) (ZRB-2002) procedures and the de Gref and Lengkeek (2018) thin-layer correction; * indicates the reported S_{V1D} values are for a depth range from 0 m to 10 m.

Note 2: The uncertainty for volumetric settlement was derived based on the sensitivity of volumetric settlement to PGA, C_{FC} , and P_L for each earthquake event for VsVp 57203 *Shirley Intermediate School* and CC LIQ 1 – CPT 5586 – *Vivian St* sites. Taking the 50th percentile as the baseline case, the minimum and maximum values corresponding to the difference between the 25th percentile and the 50th percentile and the 75th percentile and the 50th percentile were determined. The arithmetic mean of the range of the minimum and maximum difference was evaluated for each patch at the two sites. The maximum arithmetic mean for each earthquake event was rounded to the nearest five and used as the uncertainty value. Accordingly, the 1-D volumetric settlement uncertainties of ±20, ±50, ±25, and ±50 mm for the Sep-10, Feb-11, Jun-11, and Dec-11 earthquake events, respectively, were used for all sites in this study.

Table 4: Best final estimates of ejecta-induced settlement for the site.

EQ Event	10-m buffer			20-m buffer			50-m buffer		
	$S_{E,L}$ (mm)	$S_{E,P}$ (mm)	$S_{E,final}$ (mm)	$S_{E,L}$ (mm)	$S_{E,P}$ (mm)	$S_{E,final}$ (mm)	$S_{E,L}$ (mm)	$S_{E,P}$ (mm)	$S_{E,final}$ (mm)
Sep-10	ND	0	0	ND	0	0	ND	0	0
Feb-11	ND	0	0	ND	0	0	ND	0	0
Jun-11	ND	0	0	ND	0	0	ND	0	0
Dec-11	ND	0	0	ND	0	0	ND	0	0

Notes: $S_{E,L}$ = Ejecta-induced settlement based on LiDAR data was not determined (ND) due to the evident absence of ejecta at the site; $S_{E,P}$ = Ejecta-induced settlement based on ground and aerial photographs and LDAT property inspection reports; $S_{E,final}$ = Best final estimate of ejecta-induced settlement.

Note 3:

- $S_{E,final}$ for all buffers is based solely on $S_{E,P}$ for all earthquake events due to the evident absence of ejecta.
- The 200 Cashmere Road site is in the zone of severe to excessive LPI overprediction of liquefaction severity for the Sep-10 EQ and moderate to severe LPI overprediction of

liquefaction severity for the Feb-11 EQ (Maurer et al. 2014³). No liquefaction ejecta-induced damage was reported for the properties within the 50-m buffer.

Summary:

The best estimate of the ejecta-induced free-field ground settlement at the 200 Cashmere Road site for the SEP 2010, FEB 2011, JUN 2011, and DEC 2011 earthquake is 0 mm, 0 mm, 0 mm, and 0 mm, respectively.

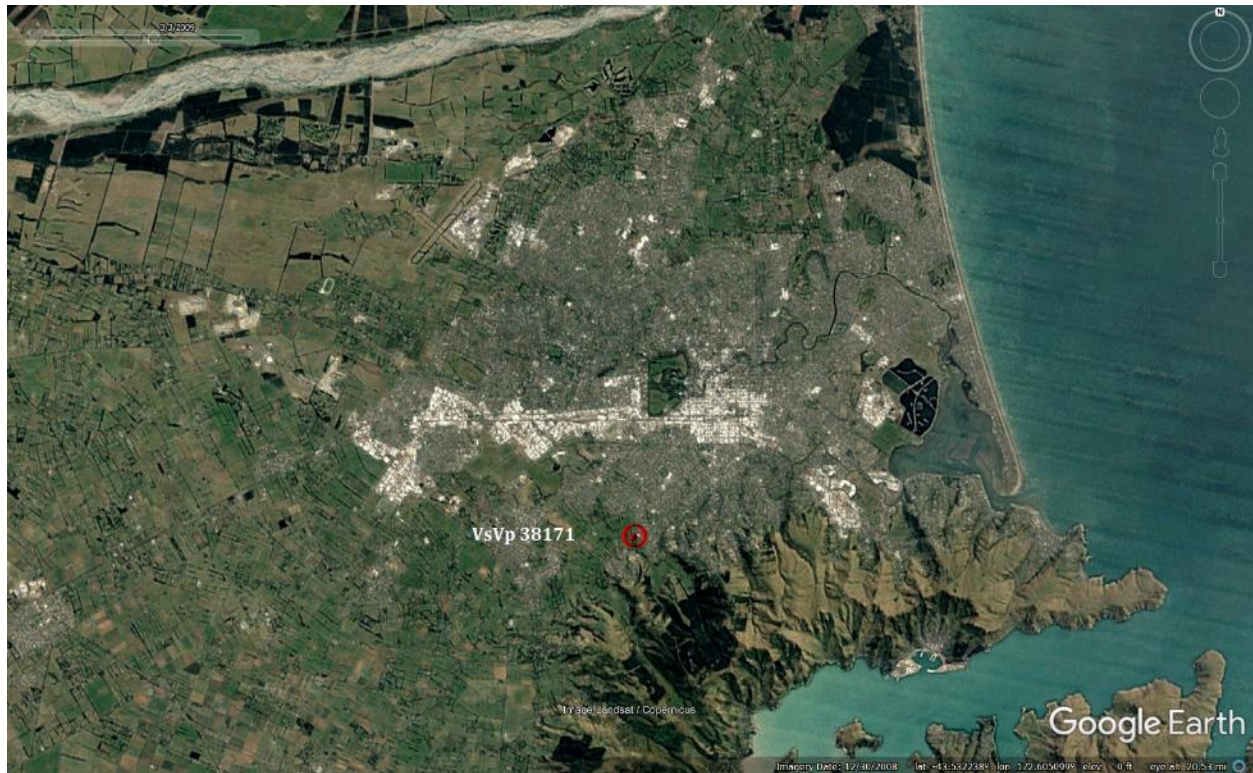


Figure 2: Location of the site.

³ Maurer, B. W., Green, R. A., Cubrinovski, M., & Bradley, B. A. (2014). Evaluation of the Liquefaction Potential Index for Assessing Liquefaction Hazard in Christchurch, New Zealand. *Journal of Geotechnical and Geoenvironmental Engineering*, 140(7), 04014032-1-11. doi:10.1061/(asce)gt.1943-5606.0001117



Figure 3: Position of the site relative to nearby buildings, vegetation, and free-face features.



Figure 4: Street view of the flat land.

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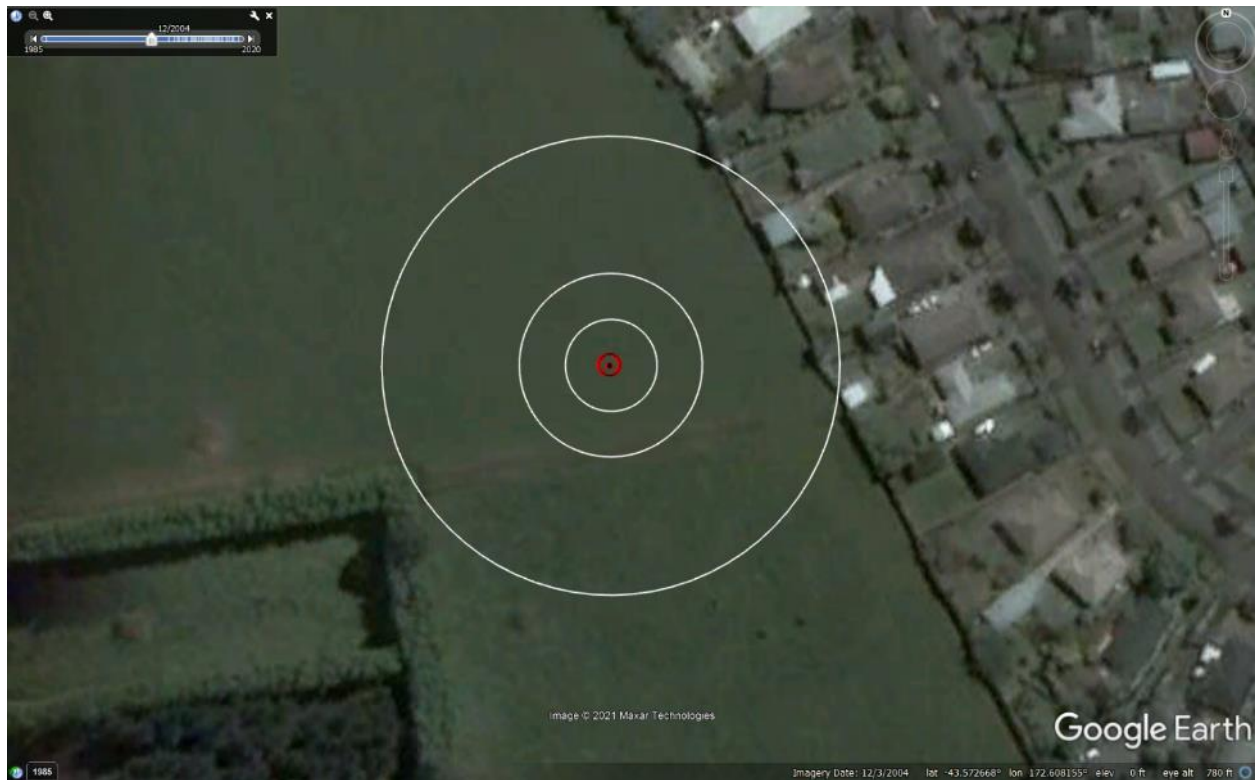


Figure 5: Satellite image of the site taken in Dec 2004.

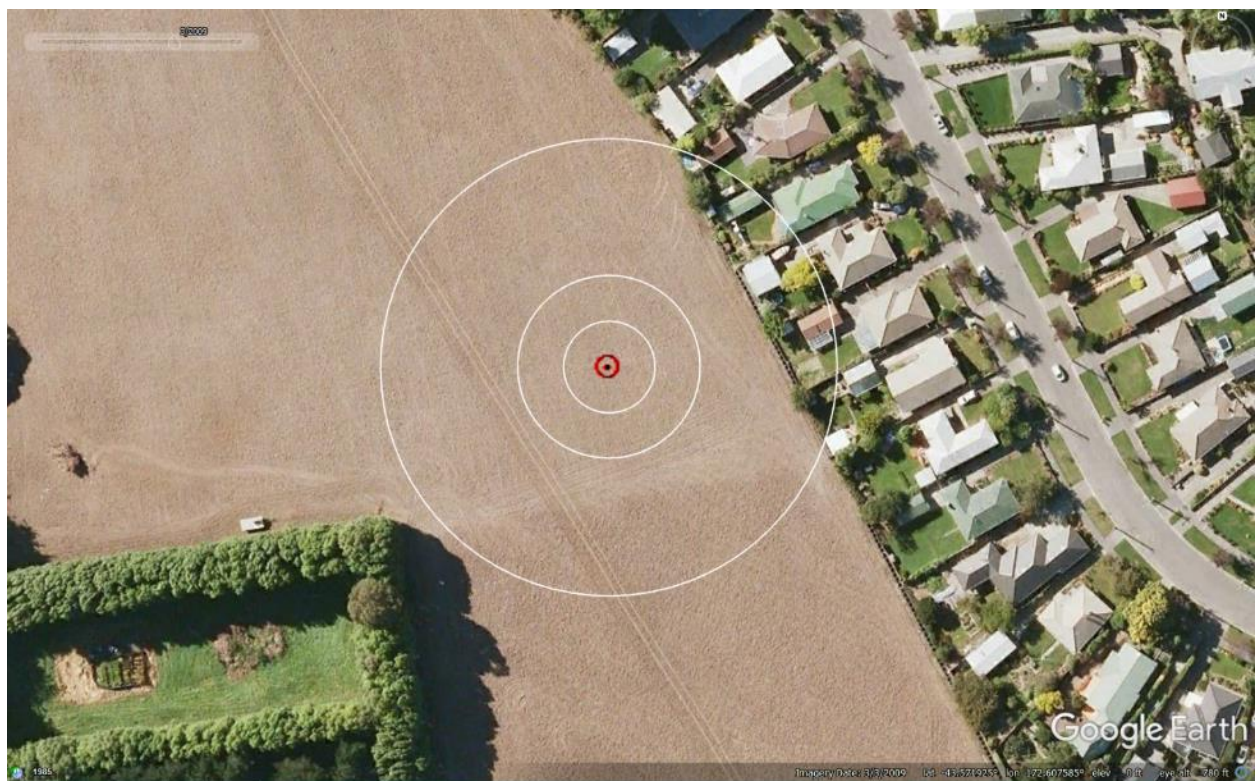


Figure 6: Satellite image of the site taken in Mar 2009.



Figure 7: Satellite image of the site taken on Sep 3, 2010.

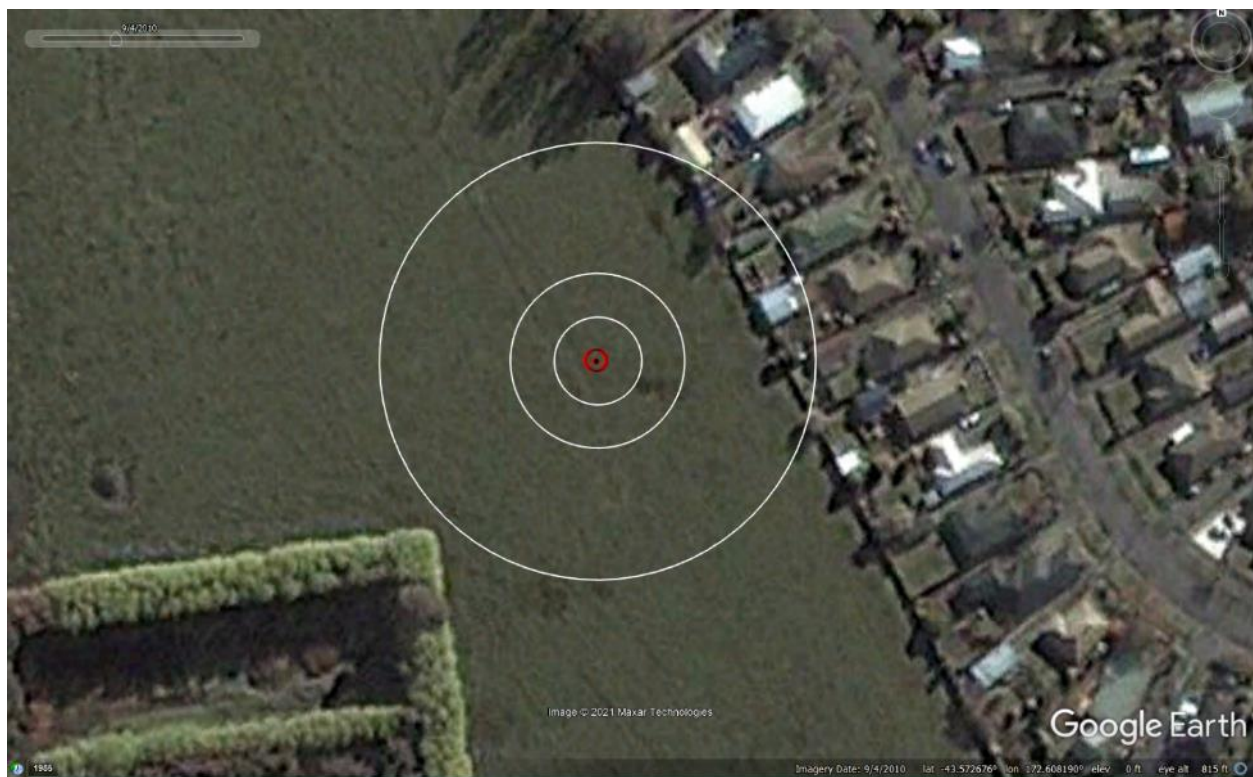


Figure 8: Satellite image of the site taken on Sep 5, 2010.



Figure 9: Satellite image of the site taken on Feb 15, 2011.



Figure 10: Satellite image of the site taken on Feb 23, 2011.



Figure 11: Satellite image of the site taken on Feb 26, 2011.



Figure 12: Satellite image of the site taken on Mar 28, 2011.

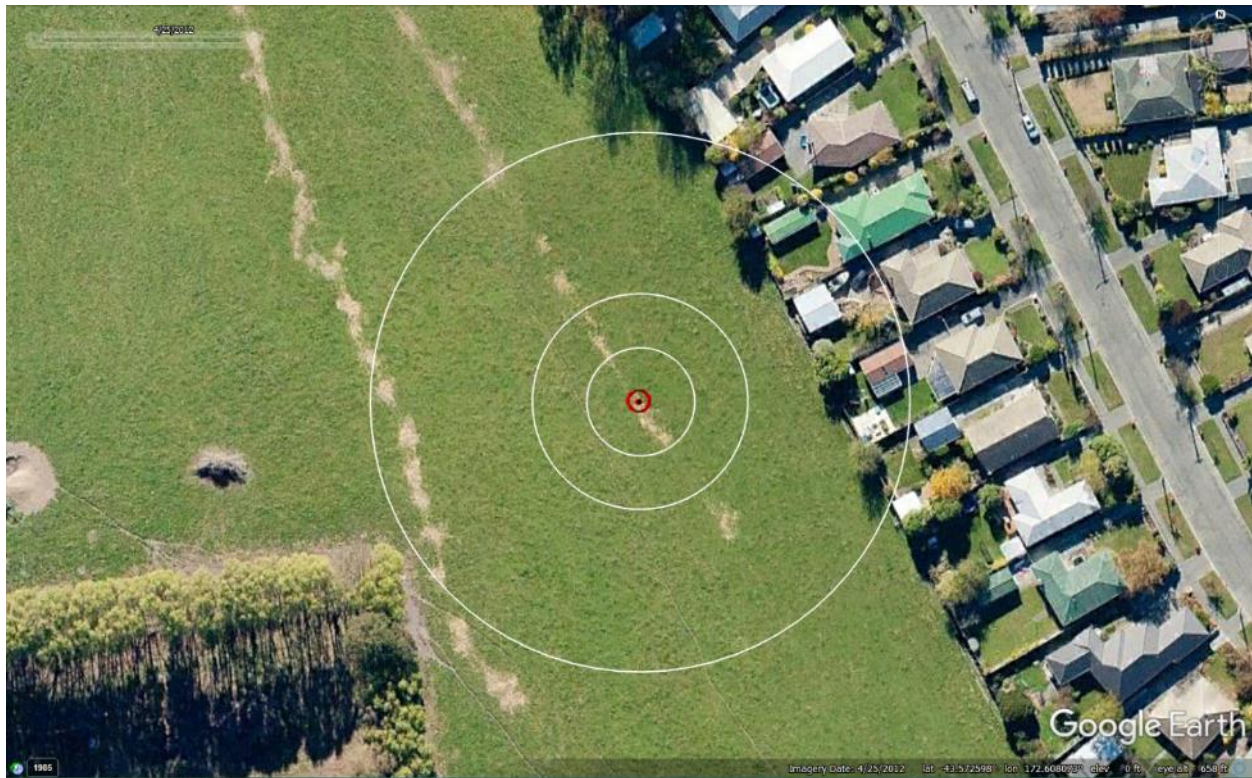


Figure 13: Satellite image of the site taken in Apr 2012.



Figure 14: Satellite image of the site taken in Nov 2015.

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Figure 15: Aerial photograph of the site from Sep 4, 2010, is not available.



Figure 16: Aerial photograph of the site taken on Feb 24, 2011.

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Figure 17: Aerial photograph of the site from June 14-15, 2011, is not available.



Figure 18: Aerial photograph of the site from June 16, 2011, is not available.

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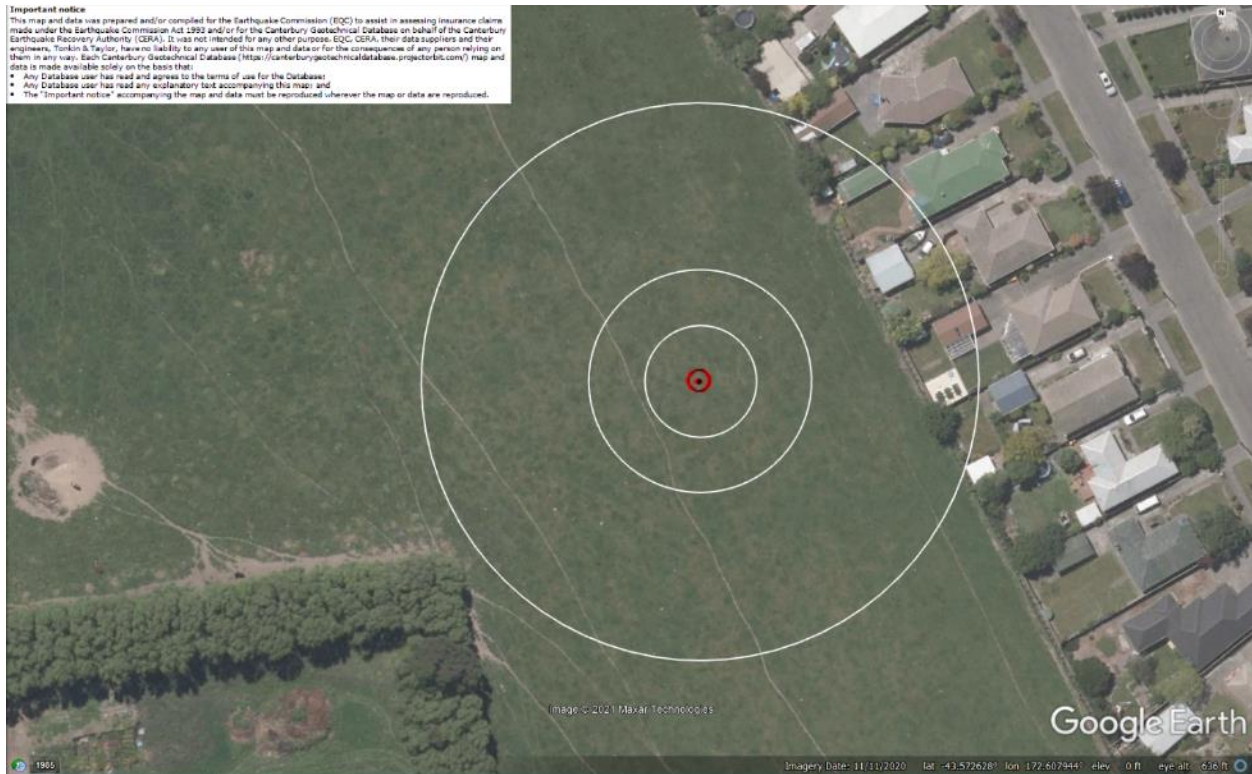


Figure 19: Aerial photograph of the site taken on Dec 24, 2011.

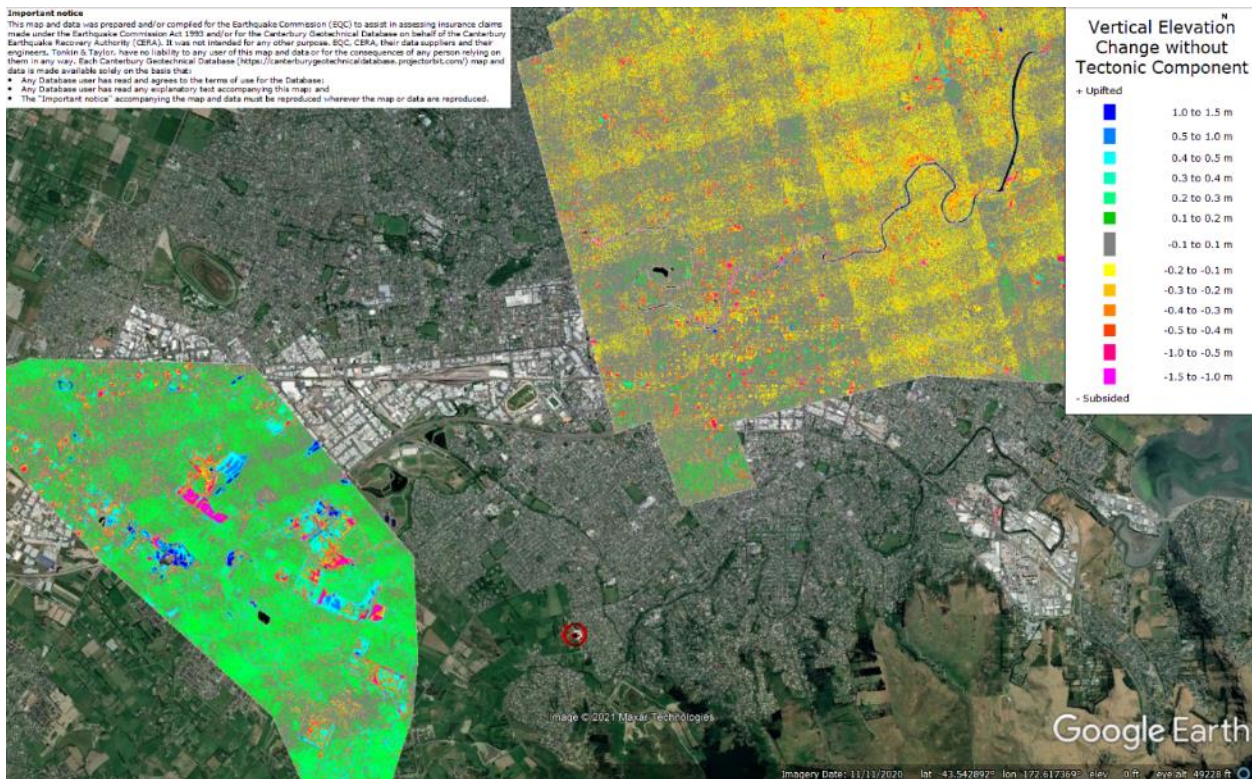


Figure 20: Vertical Ground Movements (Surface – Tectonic) for Sep 2010 Earthquake are not available.

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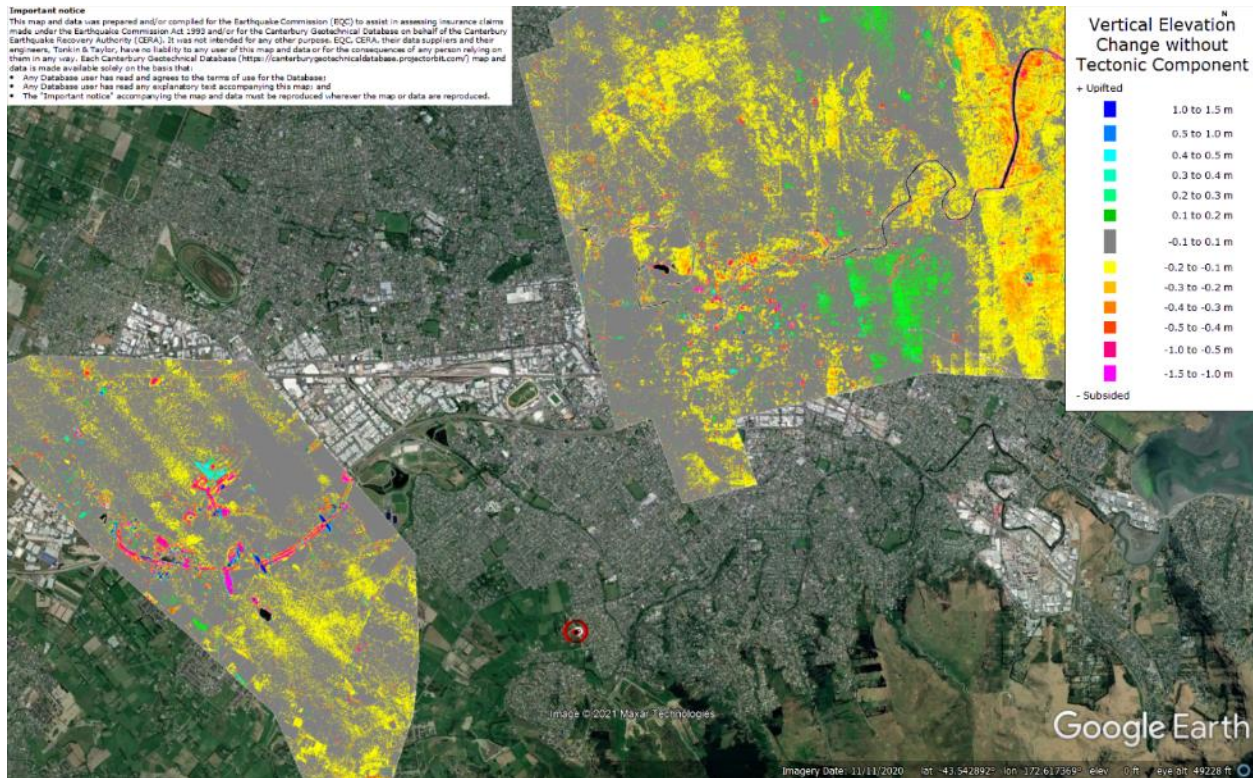


Figure 21: Vertical Ground Movements (Surface – Tectonic) for Feb 2011 Earthquake are not available.

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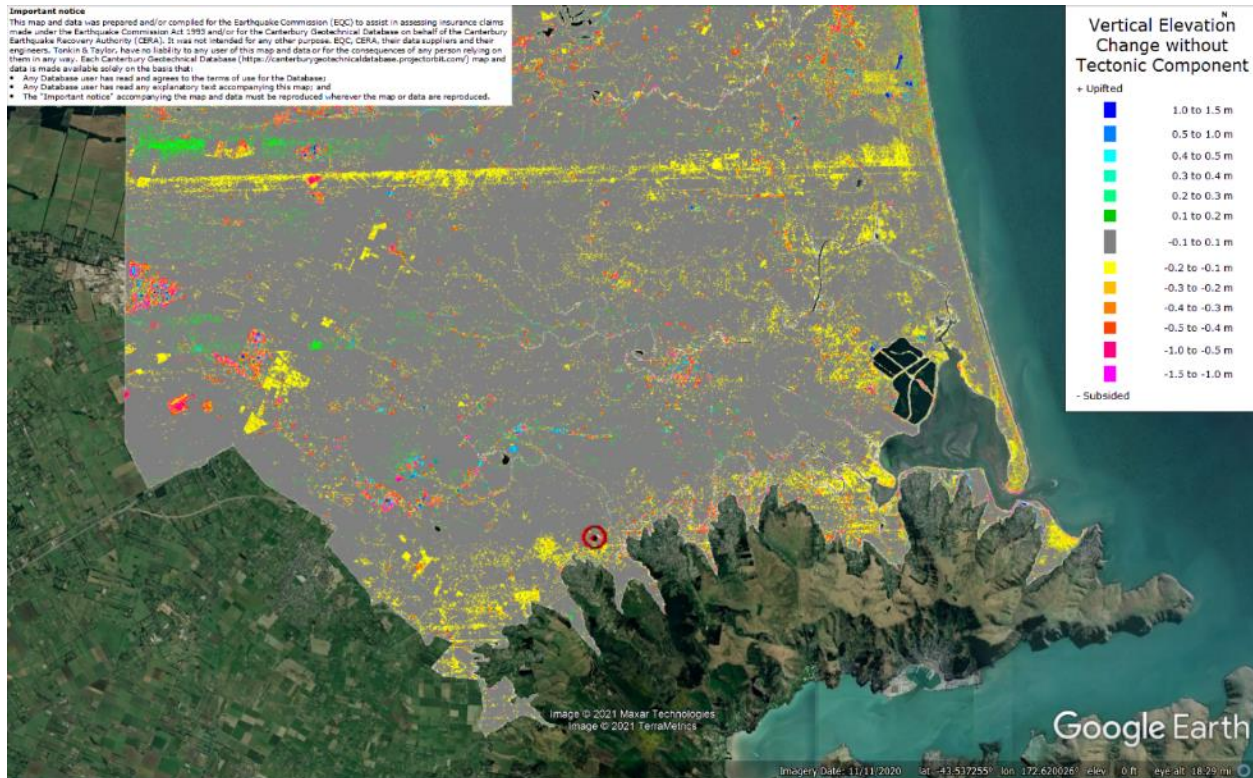


Figure 22: Vertical Ground Movements (Surface – Tectonic) for June 2011 Earthquake – the site is not in the apparent zone of overestimated ground surface subsidence.

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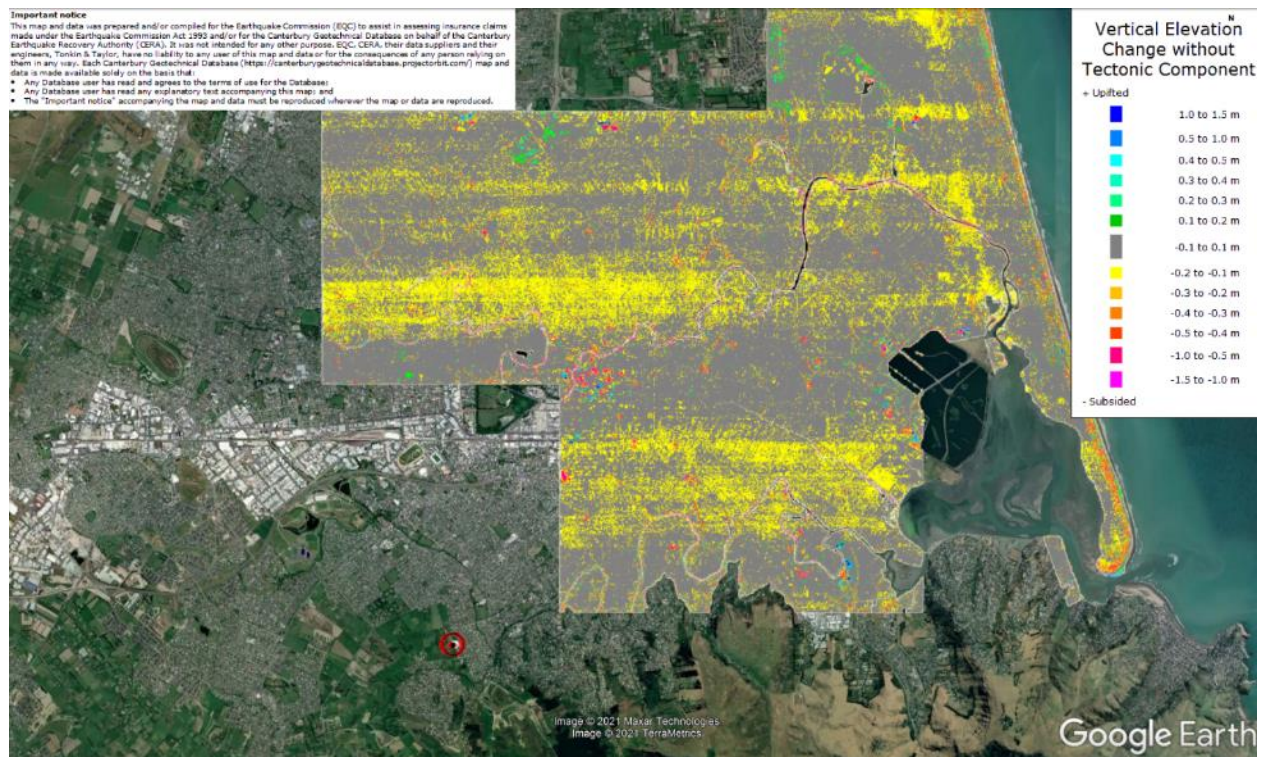


Figure 23: Vertical Ground Movements (Surface – Tectonic) for Dec 2011 Earthquake are not available.

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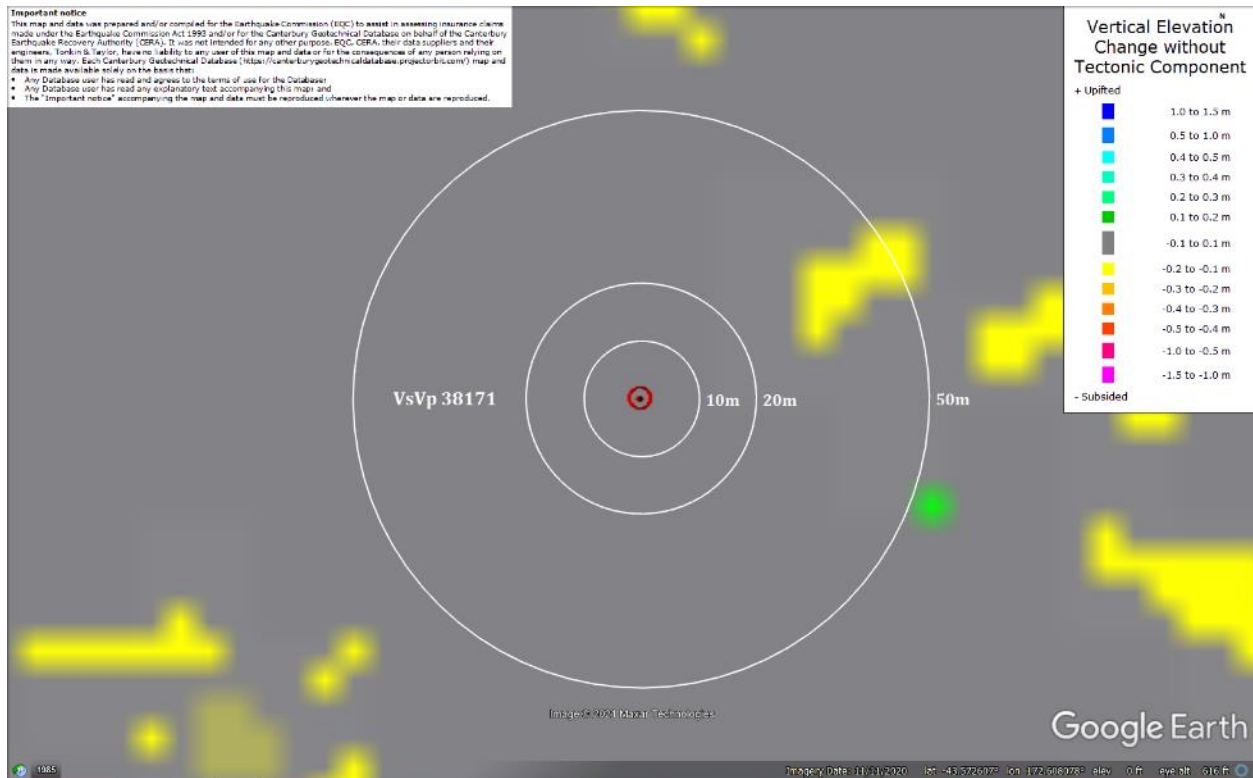


Figure 24: Ground surface subsidence without tectonic component for June 2011 Earthquake according to the LiDAR DEM.



Figure 25: No lateral spreading for Canterbury Earthquake Sequence.

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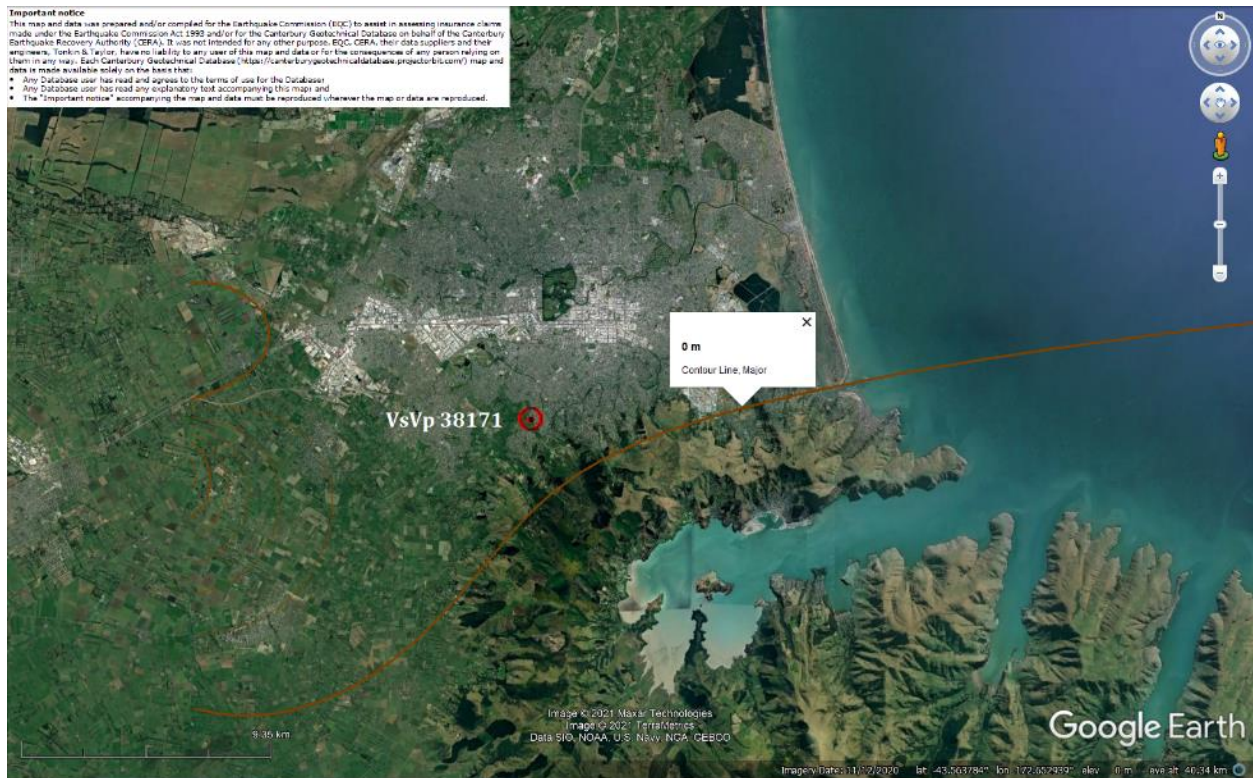


Figure 26: Vertical tectonic movements for Sep 2010 Earthquake.



Figure 27: Vertical tectonic movements for Feb 2011 Earthquake.

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Figure 28: Vertical tectonic movements for June 2011 Earthquake.



Figure 29: Vertical tectonic movements for Dec 2011 Earthquake.

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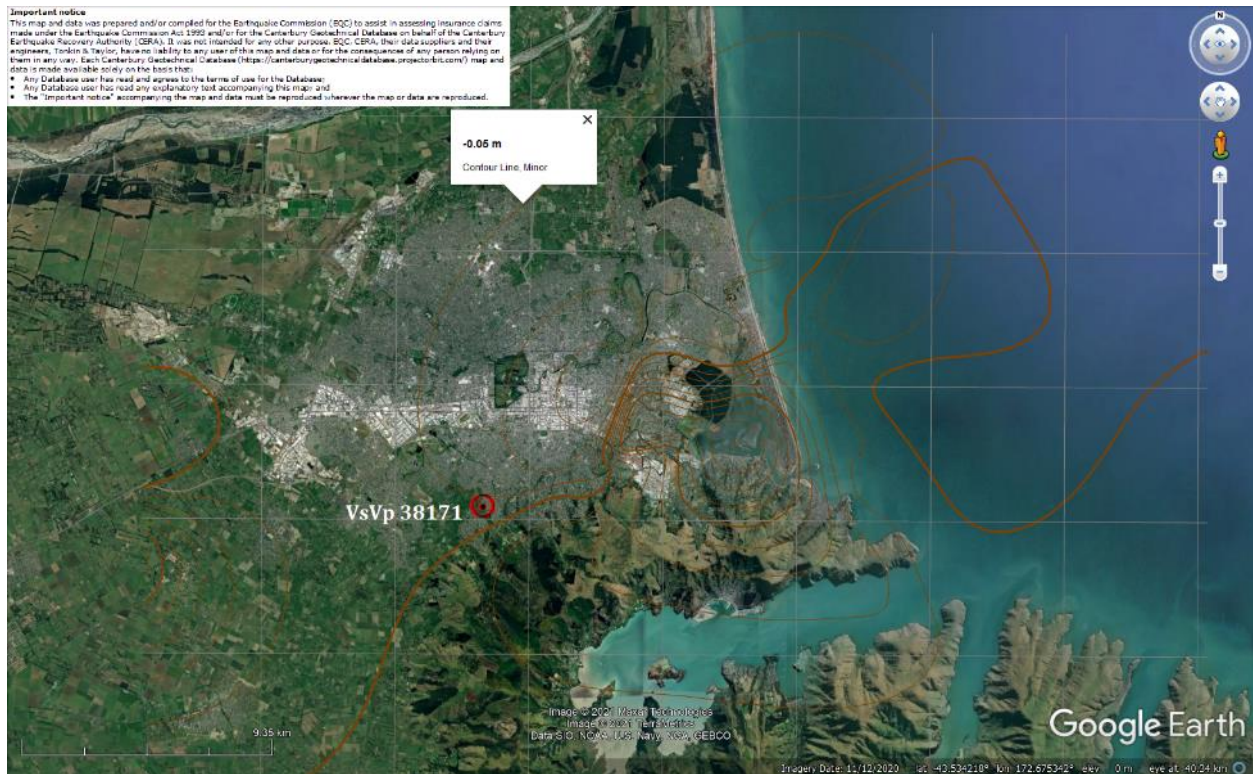


Figure 30: Vertical tectonic movements for Canterbury Earthquake Sequence.

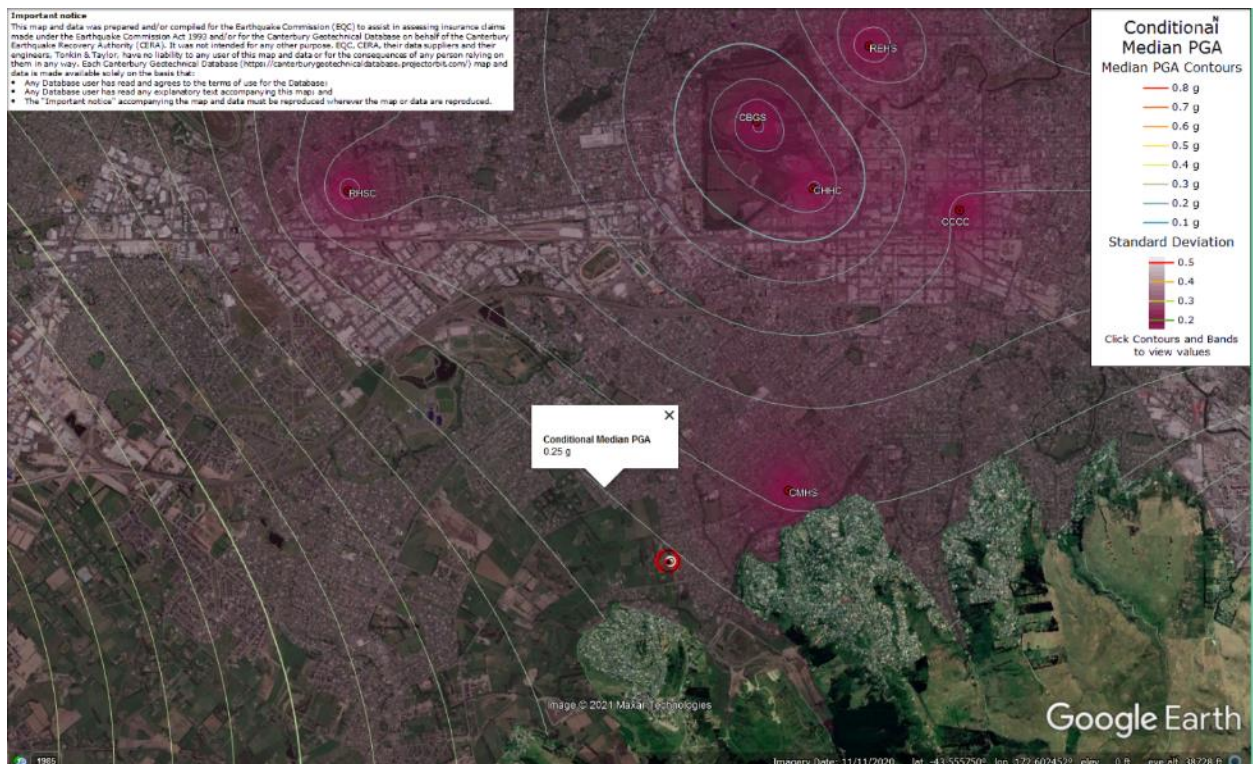


Figure 31: PGA for Sep-10 EQ (st. dev. = 0.325 to 0.350 ln units).

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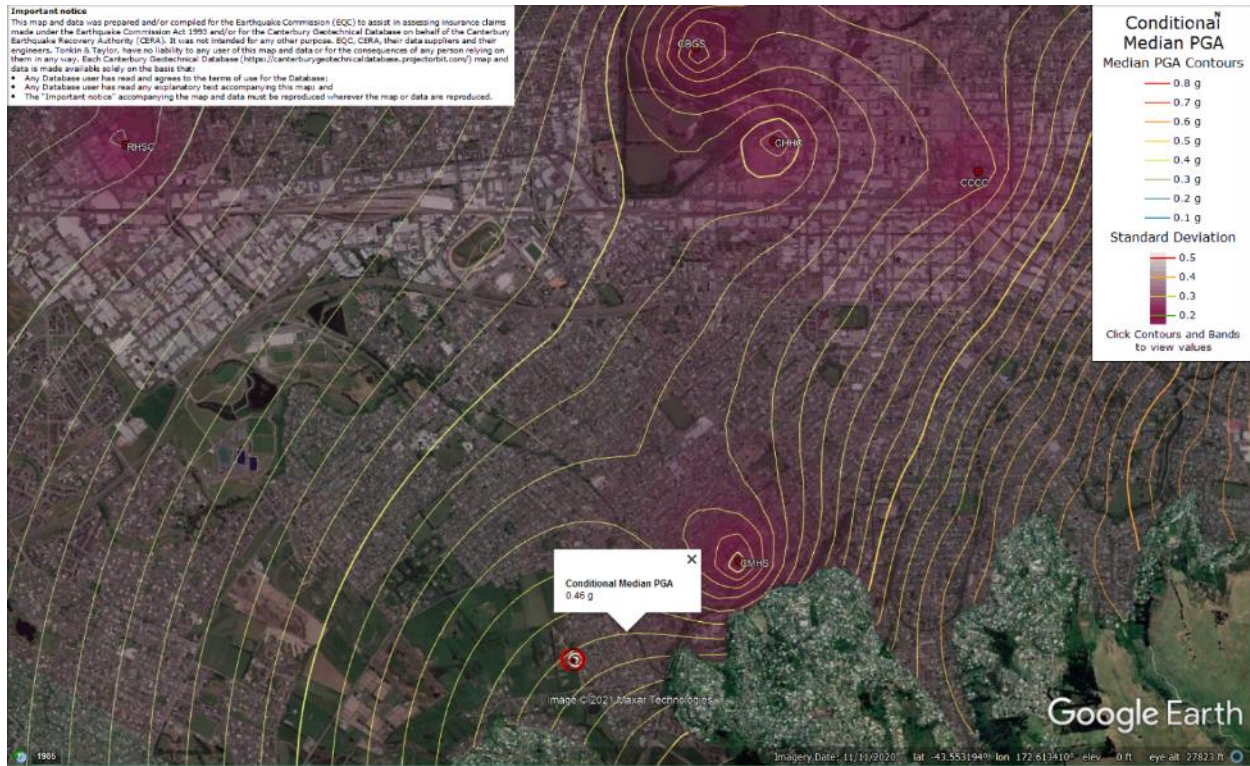


Figure 32: PGA for Feb-11 EQ (st. dev. = 0.350 to 0.375 ln units).



Figure 33: PGA for Jun-11 EQ (st. dev. = 0.375 to 0.400 ln units).

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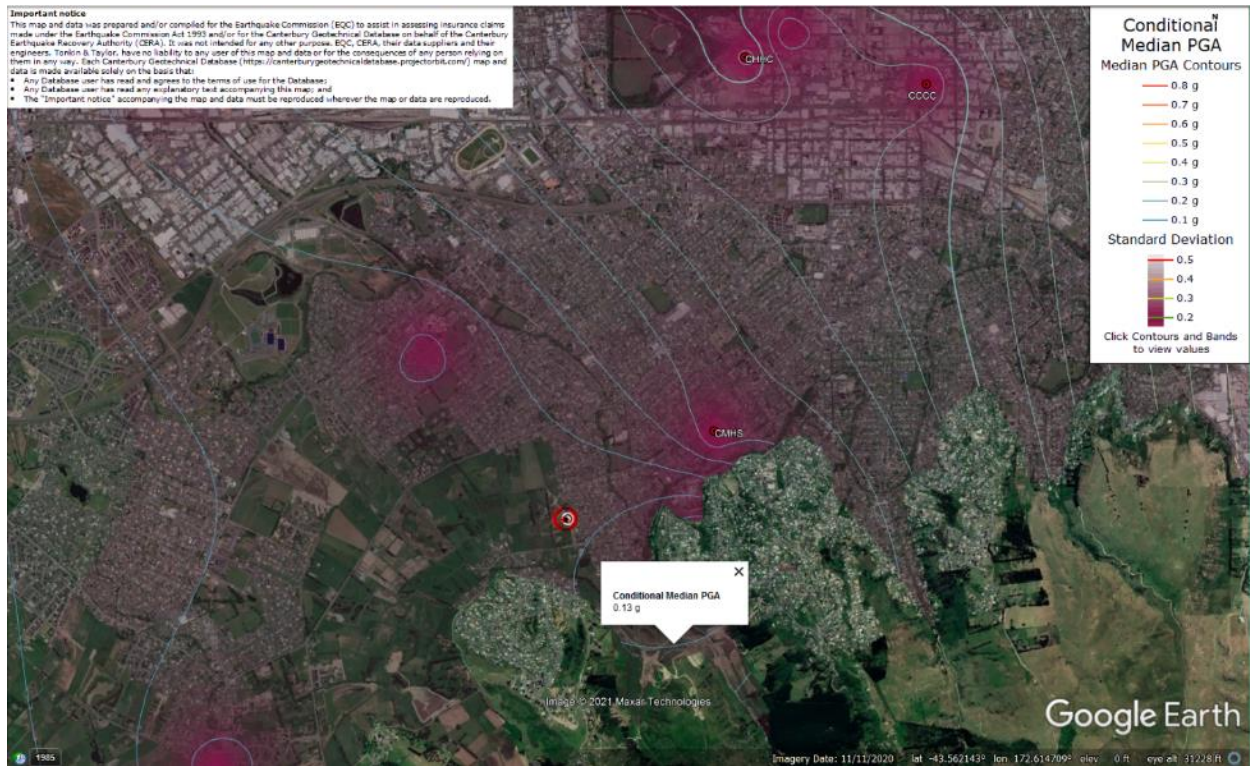


Figure 34: PGA for Dec-11 EQ (st. dev. = 0.350 to 0.375 ln units).

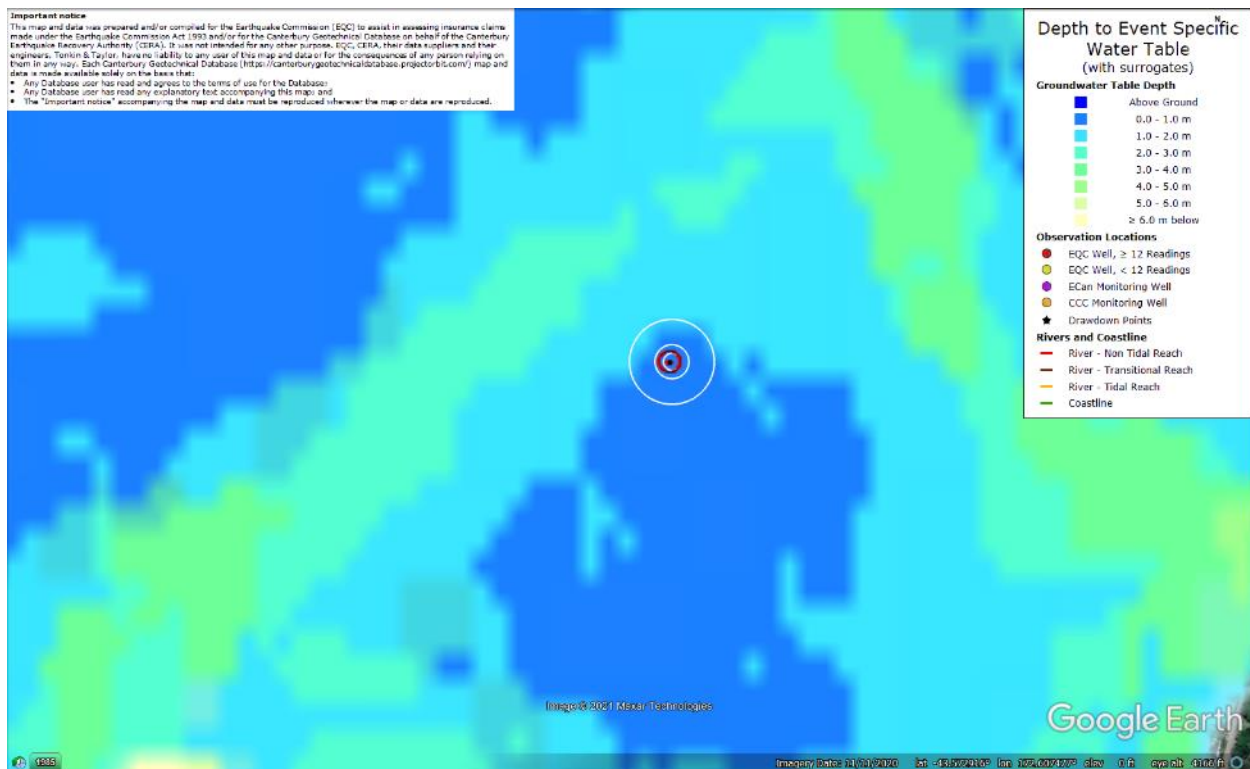


Figure 35: Depth to groundwater table for Sep-10 EQ.

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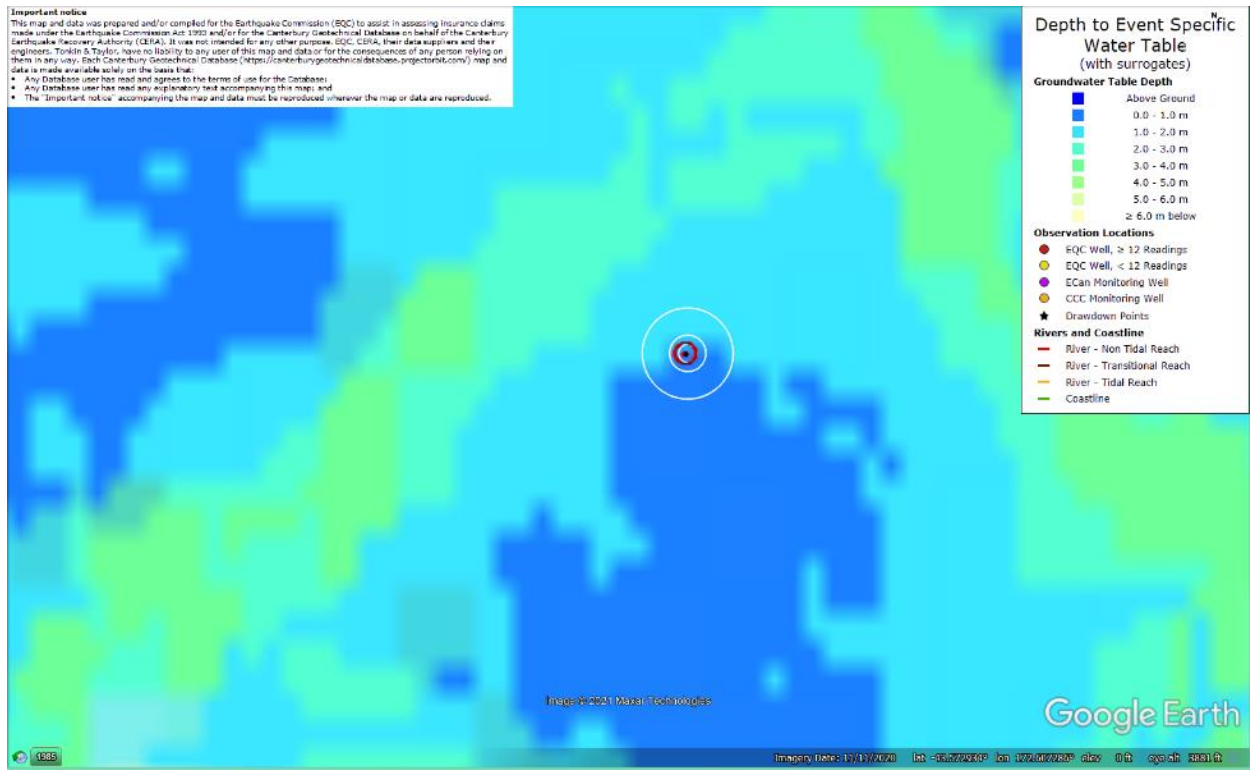


Figure 36: Depth to groundwater table for Feb-11 EQ.

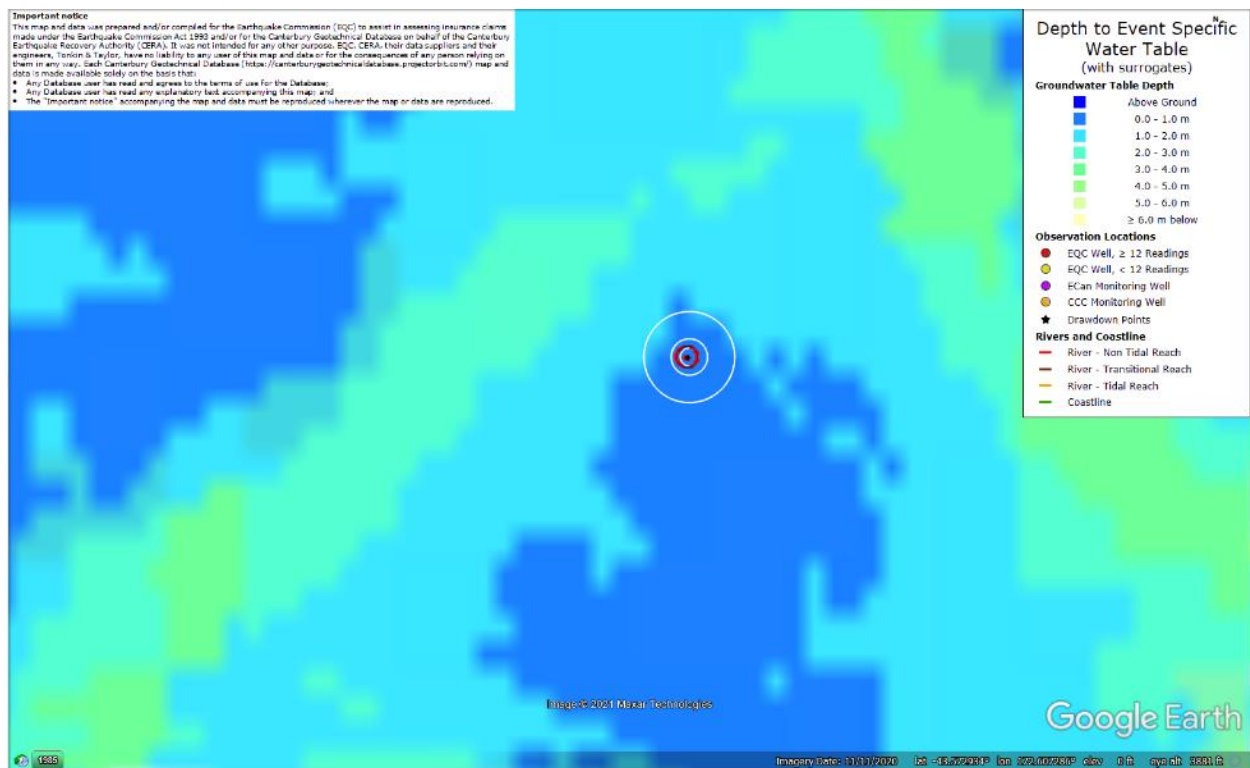


Figure 37: Depth to groundwater table for Jun-11 EQ.

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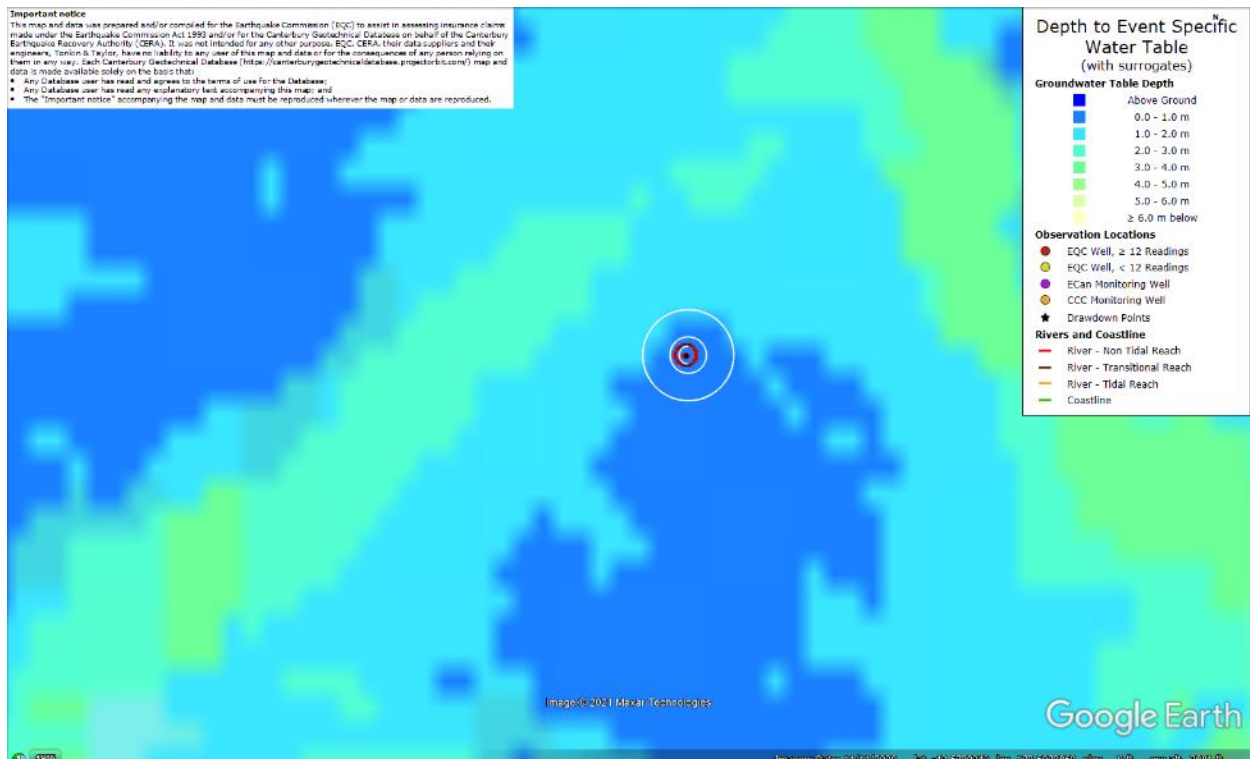


Figure 38: Depth to groundwater table for Dec-11 EQ.

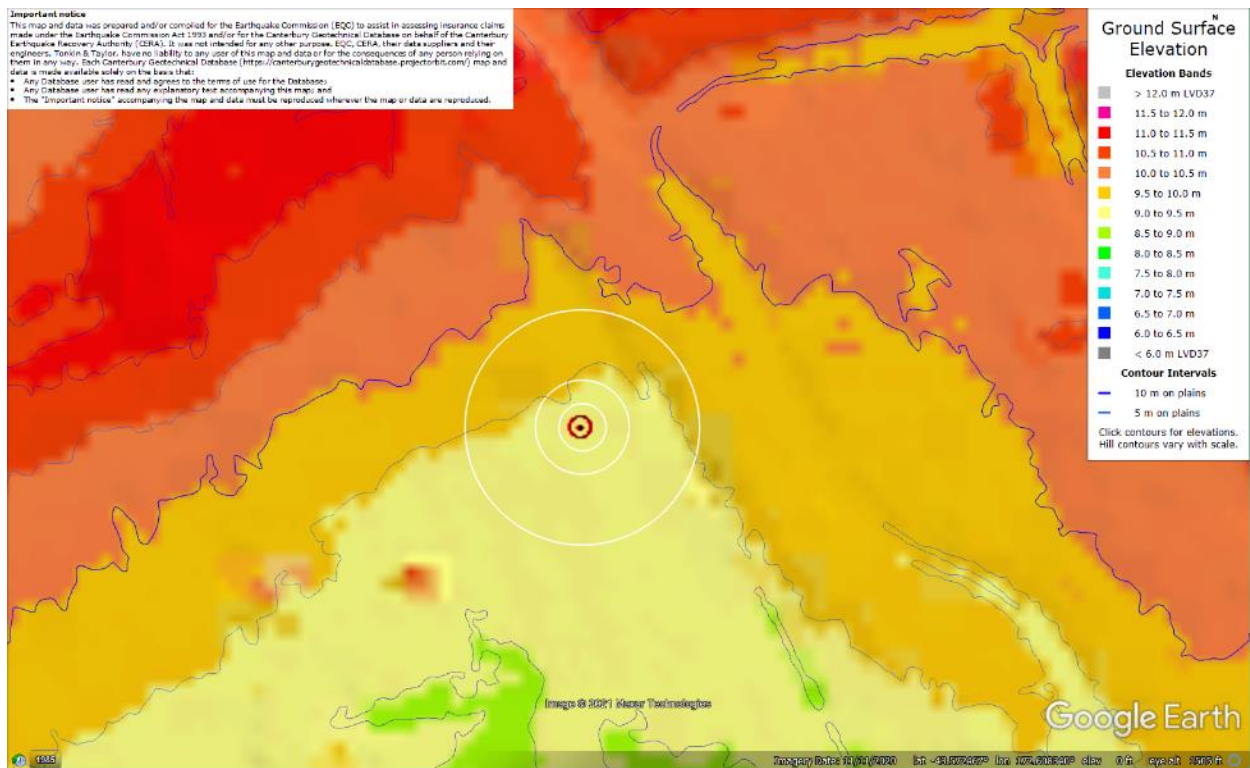


Figure 39: Ground surface elevation according to the Sep-11 LiDAR survey.

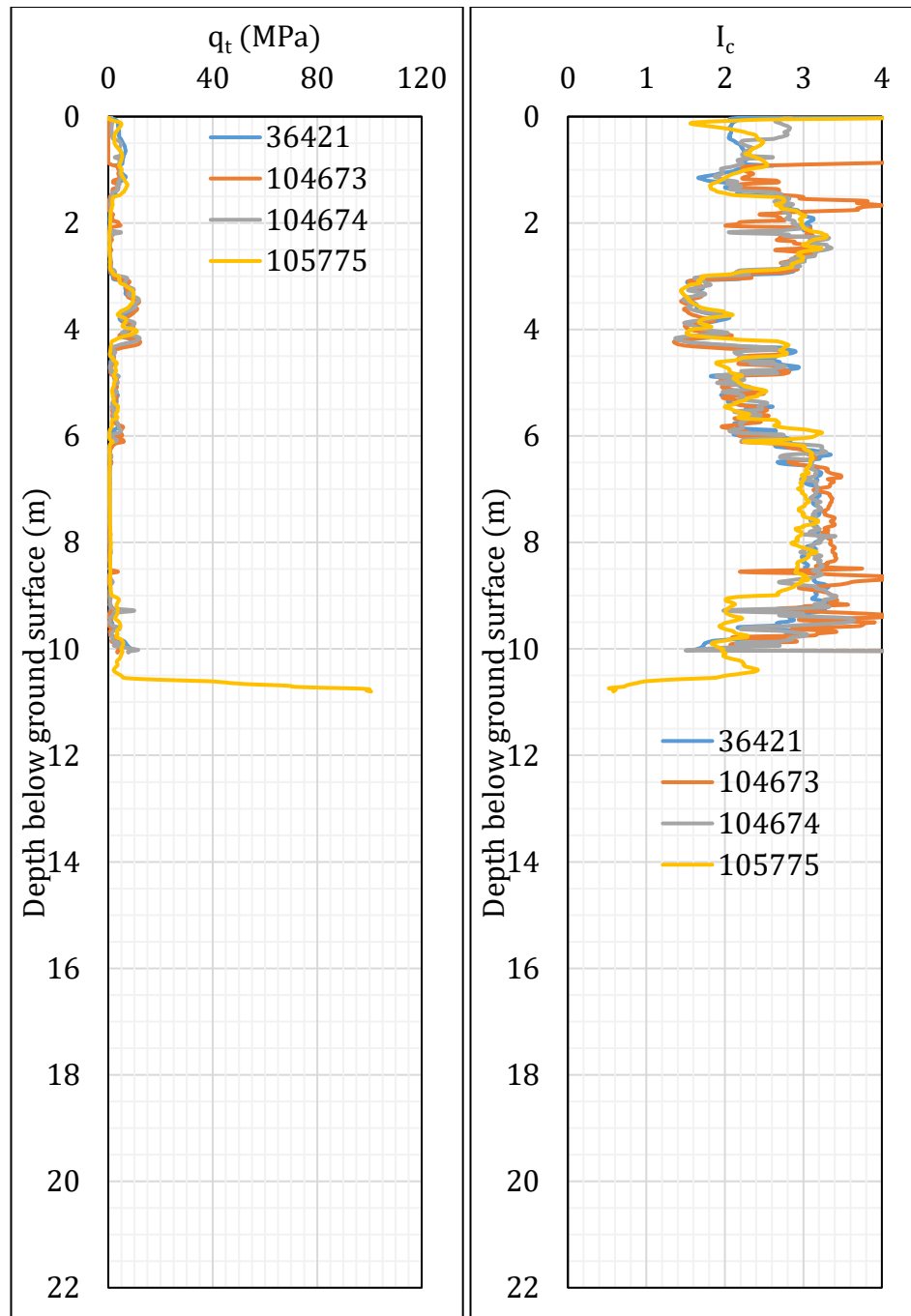


Figure 40: q_t and I_c profiles.

Note 4: The selection of CPTs for the area considered for settlement assessment (Figure 1) is based on the proximity of the CPTs to the considered areas. In accordance with that, the following table shows CPTs that were used for the volumetric settlement analysis in *Cliq v.3.0.3.2*, a CPT soil liquefaction software developed by GeoLogismiki. (The average volumetric settlements were reported in Table 8.)

Table 5: CPT profiles used in volumetric settlement analysis for areas selected for settlement assessment.

CPT ID No.	10-m buffer	20-m buffer	50-m buffer
36421 (38497)	✓	✓	✓
104673 (108752)	✓	✓	✓
104674 (108754)	✓	✓	✓
105775 (110070)	✓	✓	✓

Table 6: CPT-based results.

EQ Event	Parameter	CPT ID			
		36421	104673	104674	105775
Sep-10	S _{V1D} (mm)	64	62	65	95
	LSN	16	15	15	17
	LPI	7	7	7	10
	LPI _{ish}	4	5	5	7
	D _{FS<1} (m)	2.96	2.94	2.87	2.89
Feb-11	S _{V1D} (mm)	74	71	76	100
	LSN	20	18	19	19
	LPI	13	12	13	16
	LPI _{ish}	8	8	9	11
	D _{FS<1} (m)	1.13	2.94	2.87	2.89
Jun-11	S _{V1D} (mm)	42	41	42	76
	LSN	8	9	9	13
	LPI	2	2	2	4
	LPI _{ish}	1	0	0	0
	D _{FS<1} (m)	4.82	4.85	4.39	2.89
Dec-11	S _{V1D} (mm)	8	9	8	15
	LSN	1	2	2	3
	LPI	0	0	0	0
	LPI _{ish}	0	0	0	0
	D _{FS<1} (m)	undet.	undet.	undet.	undet.

Notes: D_{FS<1} = Depth to the first liquefiable layer (FS_L<1) that is at least 200-mm thick, as determined by the Boulanger and Idriss (2016) liquefaction-triggering procedure (P_L=50%, C_{FC}=0.13, and I_{c,cutoff}=2.6), and exported from *Cliq v.3.0.3.2*; undet. = the specified soil layer was not detected; The reported values are for a depth range from 0 m to 10 m below the ground surface.

Note 5: Based on the borehole log (BH 38197, Figure 1), the groundwater table is at a depth of 1.5 m below the ground surface. The soil profile consists of (1) topsoil to a depth of 0.1 m, (2) silt, ML, to a depth of 2.45 m, (3) silty fine sand, SM, to a depth of 3.9 m, (4) silt, ML, to a depth of 4.65 m, (5) silty fine sand, SM, to a depth of 5.35 m, (6) silt, ML, to a depth of 9.1 m, and (7) silty fine sand, SM, to a depth of 10 m (the end of the borehole). All soil layers (except the topsoil) are the Yaldhurst members of the Springston formation.